



**MASINDE MULIRO UNIVERSITY OF  
SCIENCE AND TECHNOLOGY  
(MMUST)**

MAIN CAMPUS

**UNIVERSITY EXAMINATIONS  
2021/2022 ACADEMIC YEAR**

**FOURTH YEAR SEMESTER ONE  
SUPPLEMENTARY/SPECIAL EXAMINATIONS**

**FOR THE DEGREE  
OF  
BACHELOR OF SCIENCE IN CIVIL AND STRUCTURAL  
ENGINEERING**

**COURSE CODE: CSE 411**

**COURSE TITLE: STRUCTURAL STEEL DESIGN**

**DATE: 04 OCTOBER 2022**

**TIME: 9:00-11:00 HRS**

**INSTRUCTIONS:**

1. This paper contains FIVE questions
2. Attempt ALL questions in Section A and ANY TWO questions in SECTION B
3. Marks for each question are indicated in the parenthesis.
4. You are permitted to use a printed copy of (a)BS5950 as well as (b)Steel Section Tables.  
No additional information should be written.

Examination duration is 2 Hour

MMUST observes ZERO tolerance to examination cheating

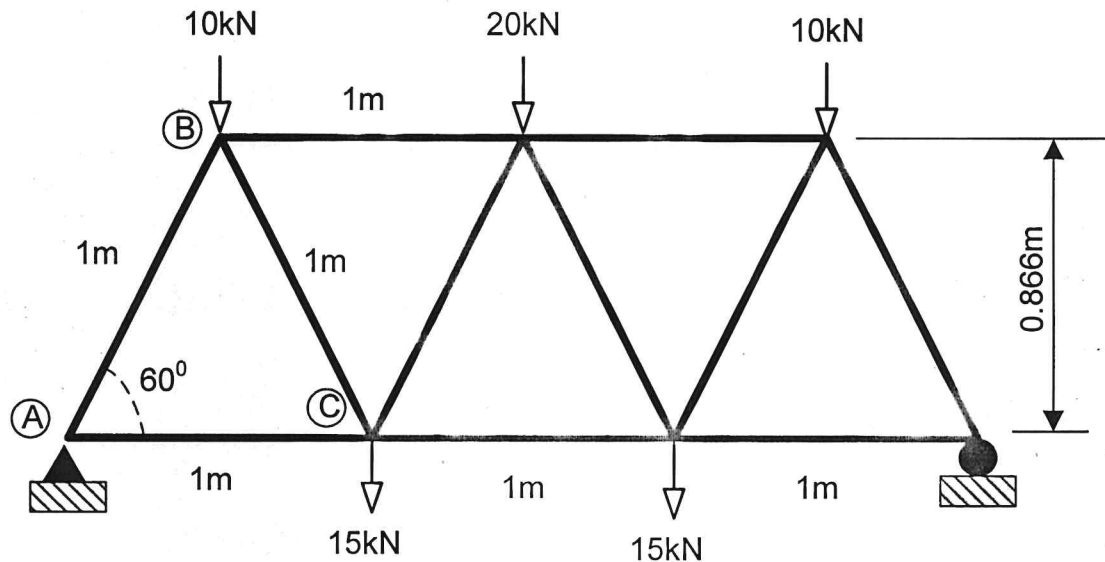
This Paper Consists of 6 Printed Pages. Please Turn Over.

## SECTION A {COMPULSORY}

### Question One (30 marks)

- a) Briefly describe the possible failure modes for bolted connections [6mks]
- b) Given the truss structure below
- Determine the adequacy of using L60x60x5 grade S355 steel section as member A-B [14mks]
  - Determine the adequacy of using L60x60x5 grade S355 steel section as member B-C [10mks]

Hint.  $L_E = 1.0 L_o$



## SECTION B (Answer Any TWO Questions)

### Question Two (20 marks)

- a) Briefly describe the failure mechanisms that a steel column may fail [4 marks]
- b) Determine the load capacity for a column to carry a compressive axial load made of (Rectangular Hollow Section) RHS 60x40x4 angle section of grade S355 steel. The column is 2.25m tall and is fully restrained at the bottom and pinned at the top. [16 marks]

### Question Three (20 marks)

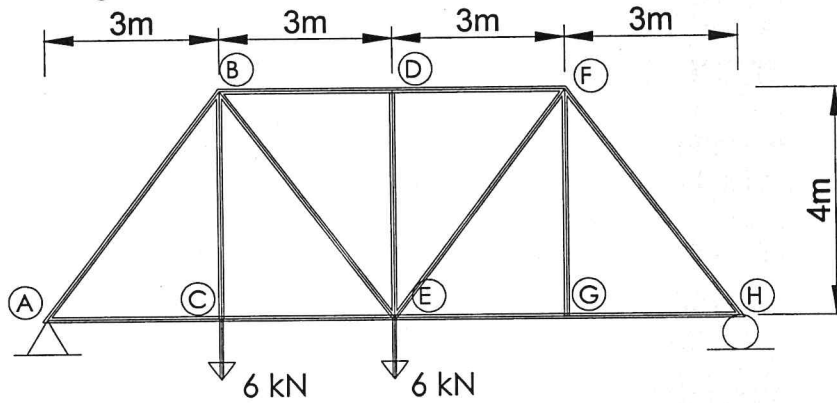
Investigate the adequacy of a 6.5 metre long simply supported Grade S275 406x140UB46 steel section loaded with uniformly distributed permanent load of 20 kN/m and variable action 7 kN/m load, if the deflection is to be limited to span/250

- Calculate and draw the shear force diagram and bending moment diagram (2 mks)
- Classify the section accordingly (3 mks)
- Check the adequacy of Moment capacity of the section (6 mks)
- Check the adequacy of shear capacity of the section (6 mks)
- Check the deflection of the member (3 marks)

**Question Four**

**(20 marks)**

Consider the following Pratt truss bridge;



Determine

(a) Calculate the reactions

(3 mks)

(b) Calculate the member forces

(17 mks)

	Member	Force, magnitude, kN	Compressive (C) or Tensile (T)
1	GH		
2	FH		
3	FG		
4	EG		
5	EF		
6	DF		
7	DE		
8	BD		
9	BC		
10	AB		
11	BE		
12	AC		
13	CE		

----- end -----

**MEMORY AIDE/ REFERENCE****i) Calculation of deflections for simple supported beams**

Deflection due to point load (P kN) at midspan of beam L m long

$$\delta = \frac{Pl^3}{48EI}$$

Deflection due to a uniformly distributed load ( $\omega$  kN/m) of a simply supported beam L meters long

$$\delta = \frac{5\omega l^4}{384EI}$$

**ii) Bolt Areas**

<b>Diameter of bolt=d (mm)</b>	12	16	20	24	30
<b>Area=A<sub>s</sub> (mm<sup>2</sup>)</b>	84.3	157	245	353	561

**iii) vii) Loading**

$$BS\ EN\ 1990-A1.3.2(4) \quad UDL_{Ultimate} = 1.35 \times G_k + 1.5 \times Q_k$$

**iv) Angle connected with one leg**With 2 bolts

$$N_{u,Rd} = \frac{\beta_2 A_{net} f_u}{\gamma_{M2}}$$

EC3-1-8

3.10.3

With 3 or more bolts

$$N_{u,Rd} = \frac{\beta_3 A_{net} f_u}{\gamma_{M2}}$$

Pitch	$p_1 \leq 2.5 d_o$	$\geq 5.0 d_o$	
2 bolts	$\beta_2$	0.4	0.7
3 bolts or more	$\beta_3$	0.5	0.7

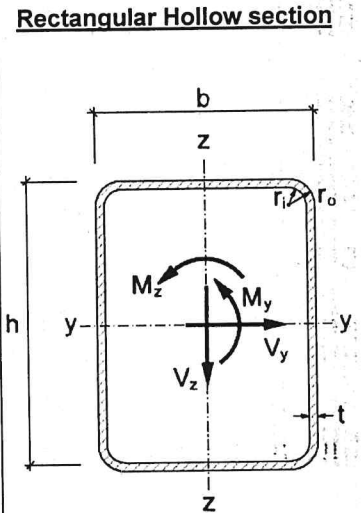
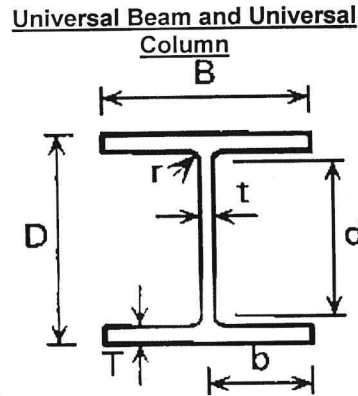
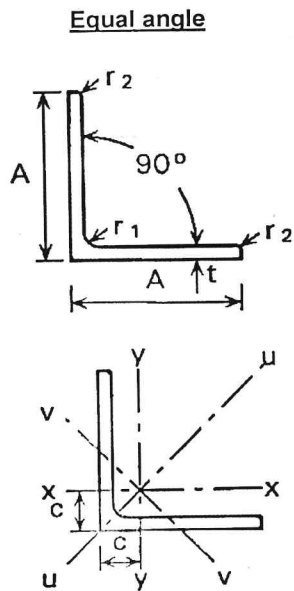
**v) Concrete compressive strength**

Concrete class	C25/30	C30/37	C35/40	C40/50
$f_{ck}$ (N/mm <sup>2</sup> )	30	37	40	50
$f_{cd}$ (N/mm <sup>2</sup> ) Design compressive strength	14.2	16.8	17	22.7

**3.1.3 Other properties**

For the elastic properties of steel, the following values should be used.

- Modulus of elasticity:  $E = 205\ 000\ N/mm^2$
- Shear modulus:  $G = E/[2(1 + \nu)]$
- Poisson's ratio:  $\nu = 0.30$
- Coefficient of linear thermal expansion  
(in the ambient temperature range):  $\alpha = 12 \times 10^{-6}$  per °C



**Section properties**

				Universal Beam	Universal Column	Equal Angle	Rectangular Hollow Section
				<b>406x140x46</b>	<b>305x305x97</b>	<b>60x60x5</b>	<b>60x40x4</b>
Mass per metre		kg/m		46.0	96.9	4.57	5.64
Depth of section		h	mm	403.2	307.9	60	60
Width of section		b	mm	142.2	305.3	60	40
		C	mm			1.64	
Thickness	Web	$t_w$	mm	6.8	9.9		
	Flange	$t_f$	mm	11.2	15.4		
		T	mm				4
Root radius		r	mm	10.2	15.2		
	root	$r_1$	mm			8	
	toe	$r_2$	mm			4	
Depth between fillets		D	mm	360.4	246.7		
Ratios for local buckling	Web	$c_w/t_w$		53.0	24.9		
	Flange	$c_f/t_f$		5.13	8.6		
		$c_w/t$					12.0
		$c_f/t$					7.00
Second moment of area	Axis y-y		cm <sup>4</sup>	15700	22200	19.4	32.8
	Axis z-z		cm <sup>4</sup>	538	7310	19.4	17.0
	Axis u-u		cm <sup>4</sup>			30.7	
	Axis v-v		cm <sup>4</sup>			8.03	
Radius of gyration	Axis y-y		cm	16.4	13.4	1.82	2.14
	Axis z-z		cm	3.03	7.69		1.54
	Axis u-u		cm			2.3	
	Axis v-v		cm			1.17	
Elastic modulus	Axis y-y		cm <sup>3</sup>	778	1450	4.45	10.9
	Axis z-z		cm <sup>3</sup>	75.7	479	4.45	8.52
Plastic modulus	Axis y-y		cm <sup>3</sup>	888	1590		13.8
	Axis z-z		cm <sup>3</sup>	118	726		10.3
Area of section	A	cm <sup>2</sup>		58.6	123	5.82	7.19

